

SO No.: 115231

Subject: VDOT BRIDGE RATING

BRIDGE NO. 1005 - 1 SPAN TRUSS

TRUSS SPLICE ANALYSIS - U2 CONNECTION

Computed By: DLN Date: 4/3/2009 Checked By: MKB Date: 4/6/2009



J:\DESIGN\VDOT Pilot Project\VDOT Trusses\01005 Gooney Crk\Gusset PL Analysis\01005 Gusset Chord Splices 040609.xls]Total Splice Plate Capacity

**TRUSS MEMBER SPLICE ANALYSIS**

For panel points along the top and bottom chord where the truss member is spliced, a reduction to the resulting force in the gusset plate is required which will account for the capacity of the splice plates.

**Summary of Truss Member Properties**

Tension = (-)  
Compression = (+)

Note: Since the bottom chord is comprised of 2 angles and has top and side splices, the top leg of the angles was broken out to determine the amount of force distributed separately to the top and side splices.

**MEMBER 4**  
DL<sub>4</sub> = 127 kips  
LL<sub>4</sub> = 100 kips  
P<sub>4</sub> = 227 kips

**MEMBER 5**  
DL<sub>5</sub> = 125 kips  
LL<sub>5</sub> = 100 kips  
P<sub>5</sub> = 225 kips

**Free Body Diagram**



P<sub>Net</sub> = 452 kips

NET FORCE AT CONNECTION RESULTS IN A COMPRESSION SPLICE

**Top Plate Properties**

t = 0.436 in. (Thickness of the Top Plate)  
W = 2.600 in. (Width of the Top Plate)  
No. of Plates = 2  
Plate Area = (t x W)  
Plate Area = (0.436 in. x 2.600 in.)  
Plate Area = 1.1336 in<sup>2</sup>  
Total Shape Area = 2.2672 in<sup>2</sup>

**Top Plate Properties**

t = 0.436 in.  
W = 2.600 in.  
No. of Plates = 2  
Plate Area = (t x W)  
Plate Area = (0.436 in. x 2.600 in.)  
Plate Area = 1.1336 in<sup>2</sup>  
Total Shape Area = 2.2672 in<sup>2</sup>

**Bottom Plate Properties**

t = 0.436 in. (Thickness of the Bottom Plate)  
W = 2.600 in. (Width of the Bottom Plate)  
No. of Plates = 2  
Plate Area = (t x W)  
Plate Area = (0.436 in. x 2.600 in.)  
Plate Area = 1.1336 in<sup>2</sup>  
Total Shape Area = 2.2672 in<sup>2</sup>

**Bottom Plate Properties**

t = 0.436 in.  
W = 2.600 in.  
No. of Plates = 2  
Plate Area = (t x W)  
Plate Area = (0.436 in. x 2.600 in.)  
Plate Area = 1.1336 in<sup>2</sup>  
Total Shape Area = 2.2672 in<sup>2</sup>

**Side Plate Properties**

t = 0.240 in. (Thickness of the Side Plate)  
W = 9.130 in. (Width of the Side Plate)  
No. of Plates = 2  
Plate Area = (t x W)  
Plate Area = (0.240 in. x 9.130 in.)  
Plate Area = 2.1912 in<sup>2</sup>  
Total Shape Area = 4.3824 in<sup>2</sup>

**Side Plate Properties**

t = 0.240 in.  
W = 9.130 in.  
No. of Plates = 2  
Plate Area = (t x W)  
Plate Area = (0.240 in. x 9.130 in.)  
Plate Area = 2.1912 in<sup>2</sup>  
Total Shape Area = 4.3824 in<sup>2</sup>

**Total Member Area Transferring Force**

8.9168 in<sup>2</sup> (Gross Section)

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8.9168 in<sup>2</sup> (Gross Section)

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**Controlling Section**

**Use: MEMBER 4**

Top Plate Distribution of Force = 25.4% of the area  
Bottom Plate Distribution of Force = 25.4% of the area  
Side Channel Distribution of Force = 49.1% of the area

*Note: Gross section properties are assumed to be sufficient to determine the distribution of forces to each shape that comprise the truss chord member. The smaller of the two members will control the force distribution*

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### Summary of Plate Properties

#### Top Splice Properties

$sp_t = 0.625$  in. (Thickness of the Splice Plate)

$W = 16.000$  in. (Width of the Splice Plate)

No. of Plates = 1

Splice Area =  $(sp_t \times W)$

Splice Area = ( 0.625 in. x 16.000 in. )

Splice Area = 10.0000 in<sup>2</sup>

**Total Splice Area = 10.0000 in<sup>2</sup>**

#### Bottom Splice Properties

$sp_t = 0.625$  in. (Thickness of the Splice Plate)

$W = 2.500$  in. (Width of the Splice Plate)

No. of Plates = 2

Splice Area =  $(sp_t \times W)$

Splice Area = ( 0.625 in. x 2.500 in. )

Splice Area = 1.5625 in<sup>2</sup>

**Total Splice Area = 3.1250 in<sup>2</sup>**

#### Side Splice Properties

$sp_t = 0.000$  in. (Thickness of the Splice Plate)

$W = 0.000$  in. (Width of the Splice Plate)

No. of Plates = 0

Splice Area =  $(sp_t \times W)$

Splice Area = ( 0.000 in. x 0.000 in. )

Splice Area = 0.0000 in<sup>2</sup>

**Total Splice Area = 0.0000 in<sup>2</sup>**

#### Gusset Plate Properties

$g_t = 0.500$  in. (Thickness of the Gusset Plate)

Chord Depth = 10.000 in. (Width of the Gusset Plate)

No. of Plates = 2

Gusset Area =  $(g_t \times W)$

Gusset Area = ( 0.500 in. x 10.000 in. )

Gusset Area = 5.0000 in<sup>2</sup>

**Total Plate Area = 10.0000 in<sup>2</sup>**

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**Plate & Force Distribution**

Top Splice Distribution = 100% of the force distributed to the Top Plate  
Top Splice Force Distribution = 25.4% of Total Member Force

Bottom Splice Distribution = 100% of the force distributed to the Top Plate  
Bottom Splice Force Distribution = 25.4% of Total Member Force

Total Area Transferring Force (Sides)= 10.0000 in<sup>2</sup> at the connection

Side Splice Distribution = 0.0% of the area  
Side Splice Force Distribution = 0.0% of Total Member Force

Gusset Plate Distribution = 100.0% of the area  
Gusset Plate Force Distribution = 49.1% of Total Member Force

**Conclusions - Tension Splice**

Based on the area of the splice plates and the effective area of the gusset plates, the resulting distribution of truss member forces to the plates is proportional to the ratio of gross effective areas of the truss member shapes and the connecting plates that connect members. Therefore the transferred applied truss member force from one chord member to the next member is:

<b><u>MEMBER 4</u></b>		<b><u>MEMBER 5</u></b>	
<b>Top Splice Plate takes</b>	<b>25.4%</b>	<b>Top Splice Plate takes</b>	<b>25.6%</b>
<b>Bottom Splice Plate takes</b>	<b>25.4%</b>	<b>Bottom Splice Plate takes</b>	<b>25.6%</b>
<b>Side Splice Plates take</b>	<b>0.0%</b>	<b>Side Splice Plates take</b>	<b>0.0%</b>
<b>Gusset Plates take</b>	<b>49.1%</b>	<b>Gusset Plates take</b>	<b>49.6%</b>

**FOR INPUT INTO MATHCAD - TENSION SPLICE**

Force Transferred by Gusset Plate GP<sub>4</sub> = NA

Force Transferred by Gusset Plate GP<sub>5</sub> = NA

Force Transferred by Fasteners BP<sub>4</sub> = NA

Force Transferred by Fasteners BP<sub>5</sub> = NA

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**Buckling Check (For Compression Splices, not required for Tension Splices)**

AASHTO Guide Specifications for Strength Design of Truss Bridges (Load Factor Design (1985))

Section 1.8 Computation of Member Capacity - Compression Members

$$\frac{P}{0.85 A_{ge} F_{cr}} \leq 1.0$$

Where:

- P<sub>Net</sub> = 452.19 kips (Total Axial Load, DL + LL)
- A<sub>ge</sub> = 10.0000 in<sup>2</sup> (Gross Effective Area - Top Splice Plate)
- A<sub>ge</sub> = 3.1250 in<sup>2</sup> (Gross Effective Area - Bottom Splice Plate)
- A<sub>ge</sub> = 0.0000 in<sup>2</sup> (Gross Effective Area - Side Splice Plate)
- F<sub>cr</sub> = See Below ksi (Critical Load per ALFD 10.54.1.1)

AASHTO 2002 Specifications 17th Edition (Load Factor Design)

Section 10.54.1.1 Maximum Capacity - Compression Members

- F<sub>y</sub> = 30.00 ksi (Yield Stress)
- E = 29000.00 ksi (Modulus of Elasticity)
- K = 0.75 (Effective length factor in the plane of buckling. For riveted, bolted, or welded end connections) ALFD 10.54.1.2
- L<sub>c</sub> = 2.50 in. (Length of the member between points of support - Top Splice Plate)
- L<sub>c</sub> = 2.50 in. (Length of the member between points of support - Bottom Splice Plate)
- L<sub>c</sub> = 0.00 in. (Length of the member between points of support - Side Splice Plate)
- I<sub>Top Splice</sub> = 0.3255 in<sup>4</sup> (Moment of Inertia of the Top Splice Plate)
- I<sub>Bottom Splice</sub> = 0.0509 in<sup>4</sup> (Moment of Inertia of the Bottom Splice Plate)
- I<sub>Side Splice</sub> = 0.0000 in<sup>4</sup> (Moment of Inertia of the Side Splice Plate)

$$r := \sqrt{\frac{I}{A}}$$

- r<sub>Top Splice</sub> = 0.1804 in. (Radius of gyration in the plane of buckling - Top Splice Plate)
- r<sub>Bottom Splice</sub> = 0.1276 in. (Radius of gyration in the plane of buckling - Bottom Splice Plate)
- r<sub>Side Splice</sub> = 0.0000 in. (Radius of gyration in the plane of buckling - Side Splice Plate)

$$\frac{KL_c}{r} = 10.39 \quad (\text{Top Splice})$$

$$\frac{KL_c}{r} = 14.70 \quad (\text{Bottom Splice}) \quad \sqrt{\frac{2 \pi^2 E}{F_y}} = 138.13$$

$$\frac{KL_c}{r} = 0.00 \quad (\text{Side Splice})$$

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$$\text{IF } \frac{KL_c}{r} \leq \sqrt{\frac{2\pi^2 E}{F_y}} \quad \text{THEN } F_{cr} := F_y \left[ 1 - \left( \frac{F_y}{4\pi^2 E} \right) \cdot \left( \frac{K \cdot L_c}{r} \right)^2 \right] \quad \text{Equation 1}$$

$$\text{IF } \frac{KL_c}{r} > \sqrt{\frac{2\pi^2 E}{F_y}} \quad \text{THEN } F_{cr} := \left[ \frac{\pi^2 E}{\left( \frac{K \cdot L_c}{r} \right)^2} \right] \quad \text{Equation 2}$$

$F_{cr \text{ Top Splice}} = 29.92 \text{ ksi}$  (Critical Load - Top Splice Plate) Equation 1  
 $F_{cr \text{ Bottom Splice}} = 29.83 \text{ ksi}$  (Critical Load - Bottom Splice Plate) Equation 1  
 $F_{cr \text{ Side Splice}} = \text{NA}$  ksi (Critical Load - Side Splice Plate) Equation 1

**Conclusions - Compression Splices**

Now, substitute  $F_{cr}$  back into the equation to determine if the plate buckles

$$\frac{P}{0.85 A_{ge} F_{cr}} \leq 1.0$$

$0.85 A_{ge} \times F_{cr}$   
 Top Splice Plate: 254.28 kips  
 Bottom Splice Plate: 79.24 kips  
 Side Splice Plate: 0.00 kips  
 Splice Capacity: 333.51 kips

Determine the % force distribution based on the maximum allowable force at buckling

**Member 4**

% Member 4 force carried by splice 73.4%  
 % Member 4 force carried by top & Bottom Splice 73.4%

**Member 5**

% Member 5 force carried by splice 74.1%  
 % Member 5 force carried by top & Bottom Splice 74.1%

**FOR INPUT INTO MATHCAD - COMPRESSION SPLICE**

Force Transferred by Gusset Plate GP <sub>4</sub> =	26.6%	Force Transferred by Gusset Plate GP <sub>5</sub> =	25.9%
Force Transferred by Fasteners BP <sub>4</sub> =	26.6%	Force Transferred by Fasteners BP <sub>5</sub> =	25.9%